



Design Aid Composite Columns

Design tables for composite columns
under cold and fire conditions

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1 | Introduction

The use of composite columns made of an external tubular section filled of concrete with an encased I-section has many advantages: rapidity of erection (no formwork, no rebars), high resistance for a limited size and good behavior under fire. The present document is aimed at reporting the simple calculation methods developed in order to evaluate the bearing capacity of composite tubular columns with an encased I-section at room temperature and under fire conditions (R60, R90 and R120).

The calculation of the resisting compression force of these columns at room temperature without eccentricities does not necessitate any new development as a complete simple method is defined in the current EN 1994-1-1.

For cases with an eccentric loading, the M-N interaction curve of the section must be calculated in order to apply the EN 1994-1-1 resistance criteria. In the present work, this M-N interaction curve has been calculated by dividing the column cross-section into a large number of stripes and by calculating through an automatic procedure the plastic M-N internal forces for a large amount of positions of the plastic neutral axis.

Finally, a similar procedure has been applied to determine the M-N interaction curve after 60 minutes, 90 minutes and 120 minutes of an ISO fire. The distribution of temperature in the section are evaluated by ignoring the surrounding steel tube and on the basis of isothermal curves defined in the EN 1992-1-2 for concrete columns. The contribution of the surrounding steel tube is also ignored in the calculation of the resistance and stiffness of the column.

The design tables are presented for several values of the buckling length: at room temperature, 2.35m, 2.52m, 2.8m, 3.36m, 3.6m and 4m are considered; at elevated temperatures, 2.35m, 2.52m and 2.8m are considered as the buckling length of a composite column is reduced to 0.7 L in these conditions (see 4.3.5 (10) in EN 1994-1-2:2005).

The three types of calculations are described in detail hereafter in example cases (Chapter 2 to 4). The design tables for all the geometries considered are given in Chapter 5. As a validation of these results, comparisons are given between values obtained from the analytical method and from numerical simulations performed with SAFIR software (see cases 5.1 to 5.4).

2 | Resistance of the composite columns at room temperature under axial compression

Geometrical and mechanical properties of the example case:

External tubular section: 559x8.8, S235 Steel Grade
Internal H-section: HD 320x245, S460M Steel Grade
C30/37 Concrete Grade
Buckling Length: 3.6 m

Calculation of the design compression resisting force:

Yield strength of the H-section:

$$t_f = 40 \text{ mm} \rightarrow f_{yk} = 430 \text{ MPa}$$

Ratio of permanent loads SLS: $N_G/N_{G+Q} = 0.8$
(assumption)

Ratio of permanent loads ULS:

$$\left(N_G/N_{G+Q}\right)_d = \frac{\gamma_G * 0.8}{\gamma_G * 0.8 + \gamma_Q * 0.2} = 0.783$$

Creep coefficient for concrete for inside conditions
(RH = 50%; $h_0 = h_{0,max}$): 1.9 (see blue line on Figure 1).

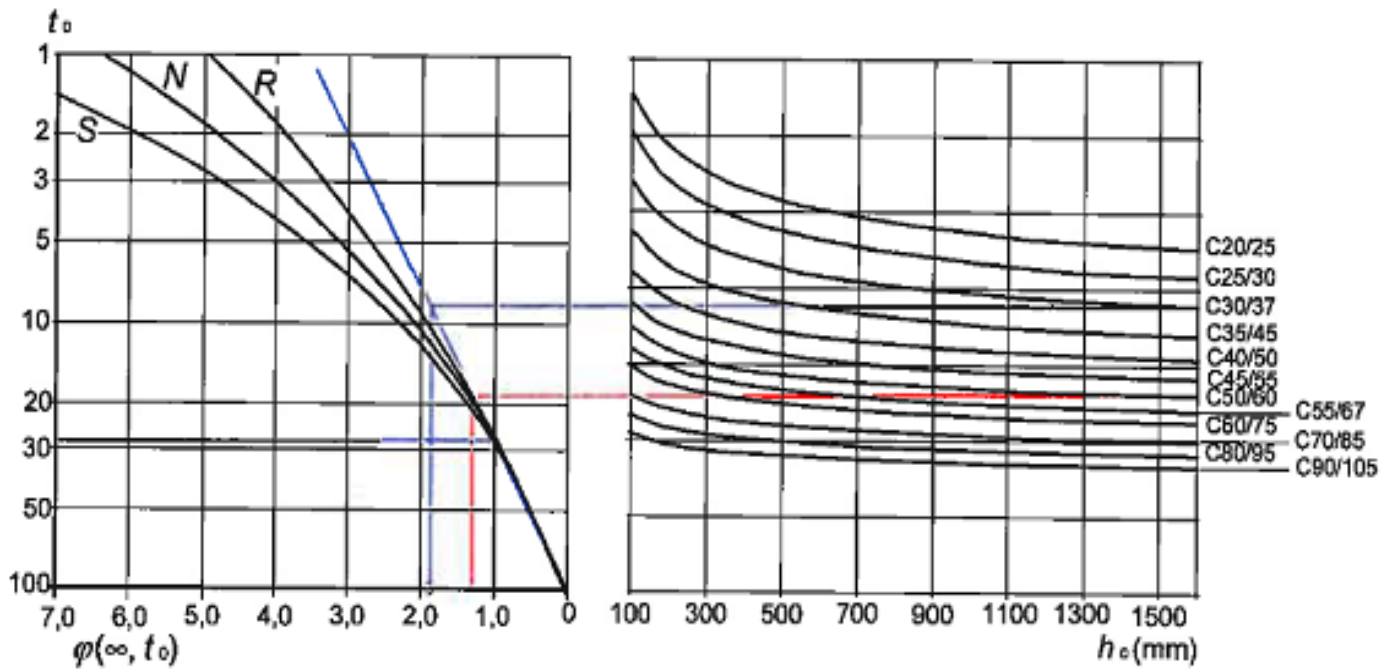


Figure 1: Method for determining creep concrete coefficient (RH = 50%) according to EN 1992-1-1:2004

Modulus of elasticity of concrete:

$$E_{c,eff} = E_{cm} \frac{1}{1 + (N_{G,Ed} / N_{Ed}) * \varphi_t} = 12867 \text{ MPa}$$

The buckling of the composite column is calculated only in the weak axis:

Effective flexural stiffness:

$$(EI)_{eff,z} = K_o (E_a I_{a,z} + K_{e,II} E_{c,eff} I_{c,z}) = 169\,342 \text{ kNm}^2$$

Coefficients K_o and $K_{e,II}$ are taken equal to 0.9 and 0.5 in order to account for member imperfections (see § 6.7.3.4. of the EN 1994-1-1:2004).

Critical normal force for weak axis:

$$N_{cr,z} = \frac{\pi^2 * EI_z}{L^2} = 128962 \text{ kN}$$

Characteristic plastic resistance in compression:

$$N_{pl,Rk} = A_a * f_{yd} + A_c * f_{cd} = 22961 \text{ kN}$$

Design plastic resistance in compression:

$$N_{pl,Rd} = A_a * f_{yd} / \gamma_{M,0} + A_c * f_{cd} / \gamma_c = 20971 \text{ kN}$$

The increase of concrete strength caused by confinement is taken into account by ignoring the 0.85 factor applied on concrete strength (applicable to all composite columns with tubular sections). However, the η coefficients applicable to circular sections (see § 6.7.3.2. (6) in EN 1994-1-1) have not been considered as a safe assumption.

Reduced slenderness in the weak axis:

$$\bar{\lambda}_z = \sqrt{\frac{N_{pl,Rk}}{N_{cr,z}}} = 0.422$$

Imperfection factor: $\alpha = 0.34$ (buckling curve b according to Table 6.5 in EN 1994-1-1:2004)

Buckling reduction factor:

$$\chi_z = \frac{1}{\phi_z + \sqrt{\phi_z^2 - \bar{\lambda}^2}} = 0.917$$

$$\text{where } \phi_z = 0.5 * [1 + \alpha(\bar{\lambda} - 0.2) + \bar{\lambda}^2] = 0.627$$

Design buckling resistance to compression:

$$N_{b,Rd} = \chi * (A_a * f_{yd} / \gamma_{M,1} + A_c * f_{cd} / \gamma_c) = 17819 \text{ kN}$$

3 | Resistance of the composite columns at room temperature under eccentric compression

The M-N interaction curve is calculated according to the procedure described hereafter:

- Discretization of the section in horizontal stripes and calculation of the cross-section area of each material included in each stripe ;
- Calculation of the plastic axial forces that each stripe can resist in tension or compression
- Calculation of internal forces M-N for a large amount of positions of the plastic neutral axis (fully plastic assumption) ;
- Drawing of the M-N interaction curve by connecting all the points of the M-N diagram.

This method necessitates the implementation of a small routine in order to obtain the non-linear M-N interaction curve. The EN 1994-1-1 proposes a simplified method for composite columns composed of an I-profile embedded in reinforced concrete: the non-linear curve is replaced by a polygonal diagram defined by four points (see § 6.7.3.2 (5) in EN 1994-1-1:2004). In the present work, the design tables will be obtained by use of the non-linear M-N interaction curve. However, the polygonal diagram is presented in the example case for quick hand-made calculations of configurations that are not included in the design tables.

Geometrical and mechanical properties of the example case:

External tubular section: 406.4x8.8, S235 Steel Grade
 Internal H-section: HEM 200, S355 Steel Grade
 C50/60 Concrete Grade
 Buckling Length: 3.36 m
 Eccentricity of the loading: $d/10 = 40.64$ mm

Calculation of the design compression resisting force:

Yield strength of the H-section:

$$t_f = 25 \text{ mm} \rightarrow f_{yk} = 355 \text{ MPa}$$

Design plastic resistance in compression:

$$N_{pl,Rd} = A_a * f_{yd} / \gamma_{M,0} + A_c * f_{cd} / \gamma_c = 10633 \text{ kN}$$

Axial force in point C:

$$N_{pm,Rd} = A_c * f_{cd} / \gamma_c = 3520 \text{ kN}$$

(assumption polygonal curve)

Bending moment associated in point C (weak axis):
 802 kNm

It should be mentioned that the point C obtained by the non-linear interaction curve is (N = 3344 kN and M = 812 kNm). Figure 2 shows that the difference between non-linear and polygonal curves is significant, especially in the weak axis that is usually the decisive one.

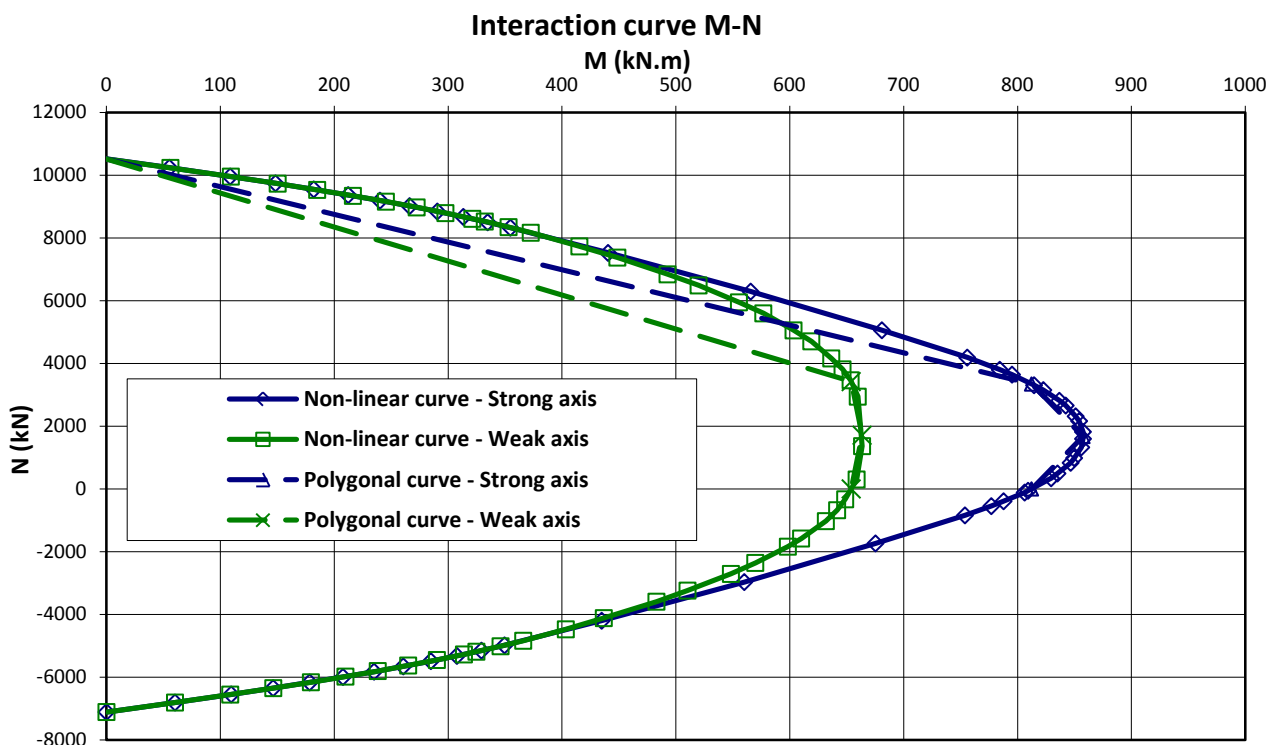


Figure 2 : M-N interaction curves for a composite column in strong and weak axis

Ratio of permanent loads SLS: $N_G/N_{G+Q} = 0.8$
(assumption)

Ratio of permanent loads ULS:

$$\left(N_G/N_{G+Q}\right)_d = \frac{\gamma_G * 0.8}{\gamma_G * 0.8 + \gamma_Q * 0.2} = 0.783$$

Creep coefficient for concrete for inside conditions
(RH = 50%; $h_0 = h_{0,max}$) : 1.4
(see blue line on Figure 1).

Modulus of elasticity of concrete:

$$E_{c,eff} = E_{cm} \frac{1}{1 + \left(N_{G,Ed}/N_{Ed}\right) * \varphi_t} = 17656 \text{ MPa}$$

Effective flexural stiffness:

$$(EI)_{eff,z} = K_o \left(E_a I_{a,z} + K_{e,II} E_{c,eff} I_{c,z}\right) = 56595 \text{ kN.m}^2$$

Coefficients K_o and $K_{e,II}$ are taken equal to 0.9 and 0.5
in order to account for member imperfections.
(see § 6.7.3.4. of the EN 1994-1-1:2004)

Critical normal force for weak axis:

$$N_{cr,z} = \frac{\pi^2 * EI_z}{L^2} = 49477 \text{ kN}$$

Total eccentricity:

$$e = e_o + e_a = d/10 + L/200 = 40.6 + 16.8 = 57.4 \text{ mm}$$

The accidental eccentricity e_a is defined according to
Table 6.5 in EN 1994-1-1:2004

First-order design bending moment (acting in weak
axis) under $N_{Sd} = 6000 \text{ kN}$ (assumption):

$$M_{z,Sd,1} = N_{Sd} * e = 344 \text{ kNm}$$

Factor accounting for second-order effects:

$$k = \frac{\beta}{1 - N_{Ed}/N_{cr,eff,z}} = 0.75 \geq 1 \rightarrow k = 1$$

Second-order design bending moment (acting in
weak axis) under $N_{Sd} = 6000 \text{ kN}$ (assumption):

$$M_{z,Sd,2} = M_{z,Sd,1} * k = 344 \text{ kNm}$$

Plastic bending moment of the section under

$$N = N_{Sd}: M_{pl,N,Rd} = 461 \text{ kNm}$$

Resistance criteria of the section in weak axis:

$$\frac{M_{Ed,z}}{M_{pl,N,Rd,z}} = 0.75 \leq \alpha_M = 0.9$$

The coefficient α_M is 0.9 for S235 and S355 steel
grades and 0.8 for S420 and S460 steel grades.
The resistance criterion shows that the resistance of
the composite columns with an eccentricity
 $e_o = d/10$ is higher than 6000 kN. An iterative
procedure allows to determine $N_{Rd} = 7224 \text{ kN}$.

4 | Resistance of the composite columns at elevated temperature

Geometrical and mechanical properties of the example case:

External tubular section: 559x8.8, S235 Steel Grade
Internal H-section: HD 320x127, S460M Steel Grade
C30/37 Concrete Grade
Buckling Length: $0.7 * 3.6 \text{ m} = 2.52 \text{ m}$
Eccentricity of the loading: $d/10 = 40.64 \text{ mm}$

4.1 | Thermal analysis

The distribution of temperature in the section has a
direct impact on the flexural stiffness and plastic
resistance of the composite column. In order to keep
the calculation method simple the temperature
profiles given in the Annex A of the EN 1992-1-
2:2004 for circular columns (Figures A-17 to A-20;
 $d = 300 \text{ mm}$) are employed as follows:

- The surrounding concrete that is submitted to a
temperature higher than 500°C is removed and
not taken into account in the mechanical
calculation. The thickness of the removed
concrete perimeter is respectively 27mm,
37mm and 50 mm for R60, R90 and R120
criteria (see Figure A-20 of EN 1992-1-2:2004).
- The temperature of the steel H-section is
considered as uniform and evaluated by use of
Figures A-17 to A-19 for the concrete cover of
the flange at a distance $b/4$ of the flange
extremity.
- The temperature of concrete without the
external hot perimeter is assumed to be equal
to the steel H-section.

This simplified method slightly overestimates
temperatures in the section: the temperature
profiles of the EN 1992-1-2:2004 are given for
300mm-large columns without steel H-section. In
larger sections, the heat used to increase the
temperature of the core (steel H-section and
concrete) leads to significantly lower temperatures
than those predicted by Eurocode temperature
profiles. This effect is more significant for large
columns, massive steel profiles and long fire
exposures.

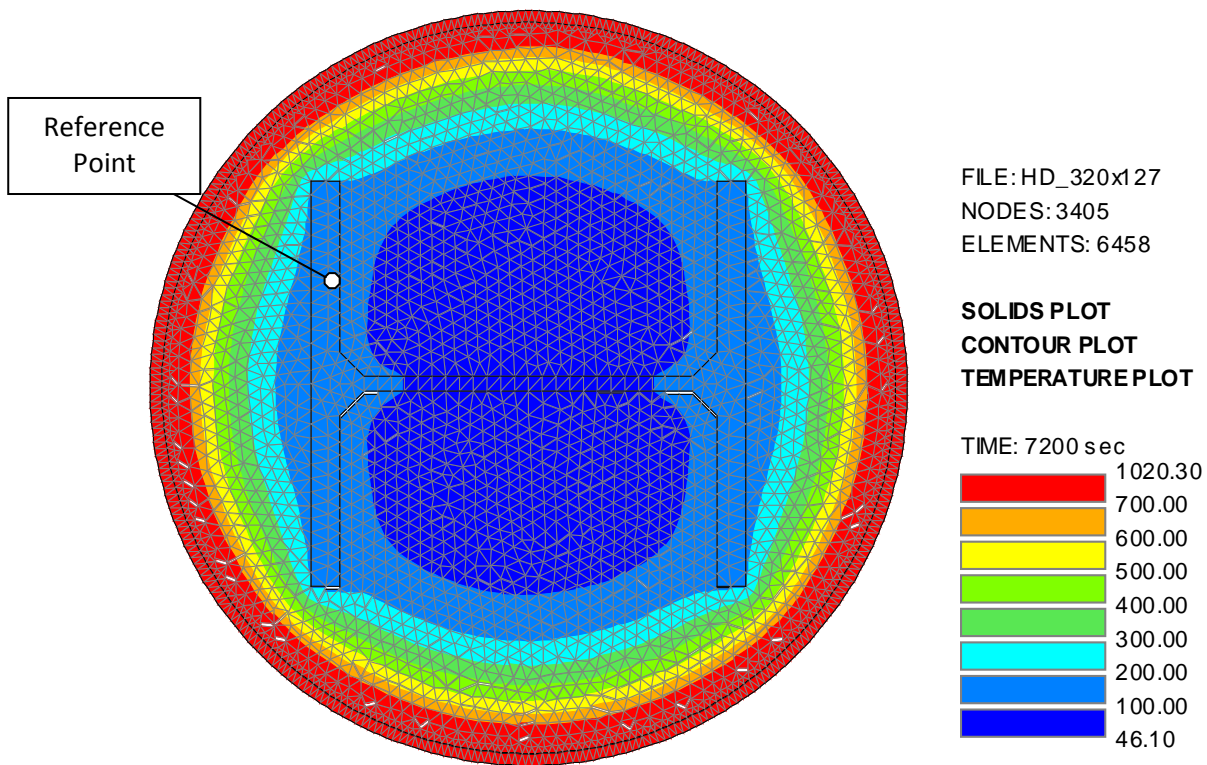


Figure 3: Distribution of temperature in a 559 mm-large composite column after 120 minutes

In the present example case, a numerical thermal simulation performed with SAFIR software respectively leads to temperatures equal to 68°C (instead of 84°C), 113°C (instead of 143°C) and 158°C (instead of 272°C) after 60, 90 and 120 minutes of ISO-fire.

Assumptions of the SAFIR simulations:

Coefficient of convection on heated surfaces:
 25 W/mK

Coefficient of convection on cold surfaces: 4 W/mK

Emissivity of steel: 0.7

Specific mass of concrete (including moisture content): 2400 kg/m³

Water content of concrete: 96 kg/m³ (4%)

Conductivity of concrete: lower limit defined in EN 1992-1-2:2004 § 3.3.3.

4.2 | Mechanical analysis

At elevated temperature, no calculation method are given in the last version of the Eurocodes for the resistance of composite columns made of a concrete-filled tubular section with an embedded H-section under axial or eccentric compression forces. Indeed, the informative Annex H dealing with this type of composite columns has been rejected by the European authorities because leading to unsafe results.

The calculation method presented hereafter and used to determine the design tables is based on the method for columns under eccentric loads at room temperature and recommendations of the European norms dedicated to the calculation of steel and concrete structures under fire (EN 1991-1-2:2002, EN 1992-1-2:2004, EN 1993-1-2:2005 and EN 1994-1-2:2005).

The M-N interaction curves of the residual section (without steel tube and external concrete ring) after 60, 90 and 120 minutes of ISO fire are calculated in a similar way to what has been done at room temperature. However, the stress-strain diagrams of steel and concrete defined in the Eurocodes at elevated temperatures are non-linear. The following assumptions have been made as simplifications:

The simplified stress-strain diagram of steel is elastic-plastic with a yield plateau for a stress:

$$\sigma_a = \frac{(k_{p,\theta} + k_{y,\theta}) f_{yk}}{2 * \gamma_{M,fi}}$$

The elasticity modulus considered in the calculation of the column flexural stiffness is

$$E_{a,\theta} = k_{E,\theta} * E_{a,20^\circ C} \text{ (see Figure 4).}$$

The simplified stress-strain diagram of concrete is elastic-plastic with a yield plateau for a stress:

$$\sigma_c = \frac{0.85 f_{c,\theta}}{\gamma_{M,fi}}$$

The elasticity modulus considered in the calculation of the column flexural stiffness is the ratio between this value σ and the strain ϵ related to this stress σ in the diagram defined in the EN 1992-1-2 (see Figure 5).

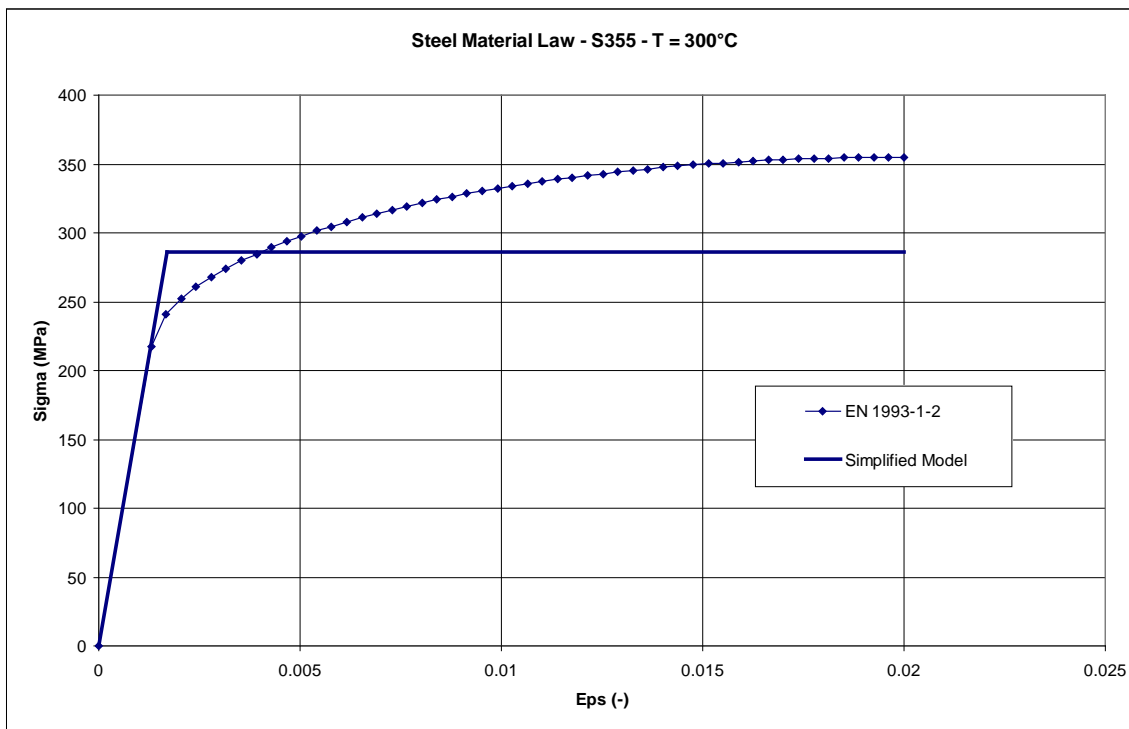


Figure 4: Comparison between EN 1993-1-2 and simplified material laws (Grade S355 – T = 300°C)

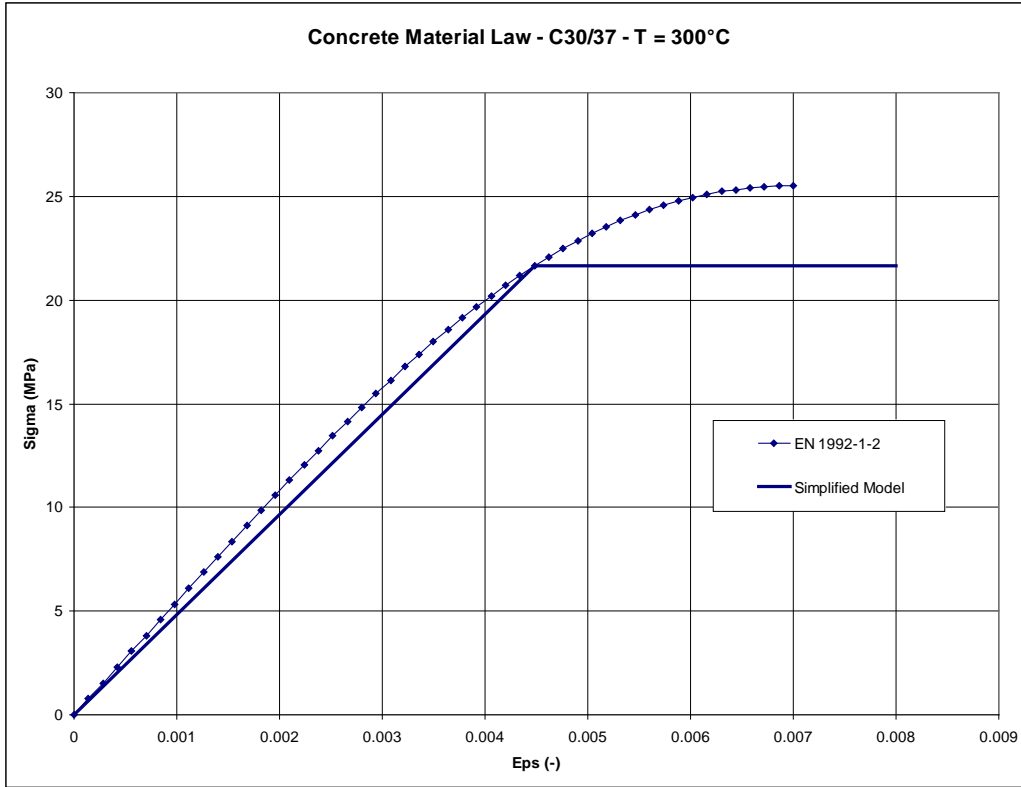


Figure 5: Comparison between EN 1992-1-2 and simplified material laws (Grade C30/37 – T = 300°C)

Calculation of the design compression resisting force:

Yield strength of the H-section: $t_f = 20.5 \text{ mm} \rightarrow f_{yk} = 440 \text{ MPa}$

$$T_{a,60} = T_{c,60} = 84^\circ\text{C} \quad ; \quad T_{a,90} = T_{c,90} = 143^\circ\text{C} \quad ; \quad T_{a,120} = T_{c,120} = 271^\circ\text{C}$$

$$\sigma_{a,60} = \frac{(k_{p,\theta} + k_{y,\theta})f_{yk}}{2 * \gamma_{M,fi}} = 440 \text{ MPa} \quad ; \quad \sigma_{a,90} = 422 \text{ MPa} \quad ; \quad \sigma_{a,120} = 367 \text{ MPa}$$

$$E_{a,60} = k_{E,\theta} E_{a,20^\circ\text{C}} = 210000 \text{ MPa} \quad ; \quad E_{a,90} = 200965 \text{ MPa} \quad ; \quad E_{a,120} = 173982 \text{ MPa}$$

$$\sigma_{c,60} = \frac{0.85 f_{c,\theta}}{\gamma_{M,fi}} = 25.5 \text{ MPa} \quad ; \quad \sigma_{c,90} = 25.0 \text{ MPa} \quad ; \quad \sigma_{c,120} = 22.4 \text{ MPa}$$

$$E_{cm,60} = k_{E,\theta} E_{a,20^\circ\text{C}} = 4091 \text{ MPa} \quad ; \quad E_{cm,90} = 4086 \text{ MPa} \quad ; \quad E_{cm,120} = 4076 \text{ MPa}$$

The calculation procedure is given in detail after 120 minutes of an ISO fire. This procedure is absolutely similar for 60 or 90 minutes.

Design plastic resistance in compression:

$$N_{pl,120,Rd} = \frac{A_a * \sigma_{a,120}}{\gamma_{M,fi}} + \frac{A_c * \sigma_{c,120}}{\gamma_{c,fi}} = 8527 \text{ kN}$$

Axial force in point C:

$$N_{pm,Rd} = A_c * f_{cd} / \gamma_c = 2606 \text{ kN}$$

(assumption polygonal curve)

Bending moment associated in point C (weak axis):
484 kNm

It should be mentioned that the point C obtained by the non-linear interaction curve is (N = 2988 kN and M = 483 kNm). Figure 6 shows that the difference between non-linear and polygonal curves is again significant after 120 minutes ISO fire in the weak axis. Figure 7 plots the non-linear curves obtained at room and elevated temperatures for the section considered in this example case in the weak axis.

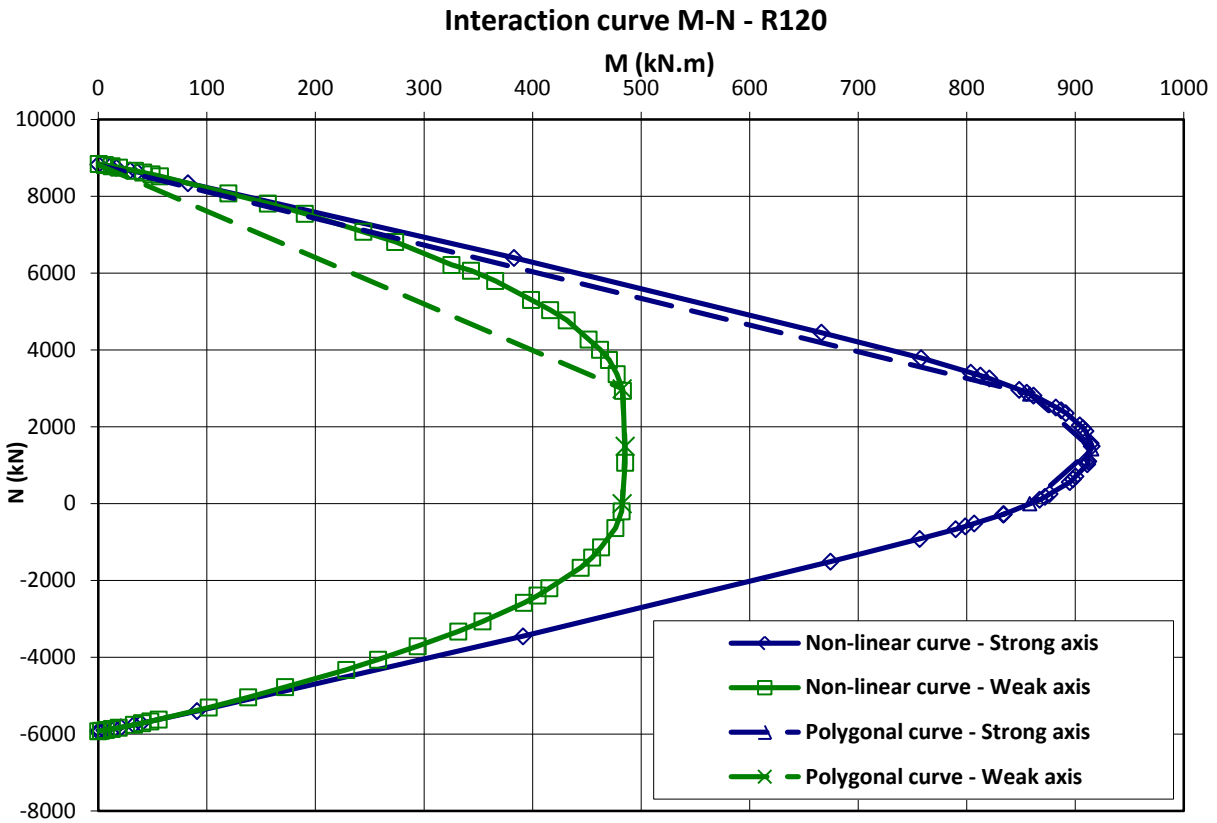


Figure 6 : M-N interaction curves for composite column in strong and weak axis after 120 minutes ISO fire

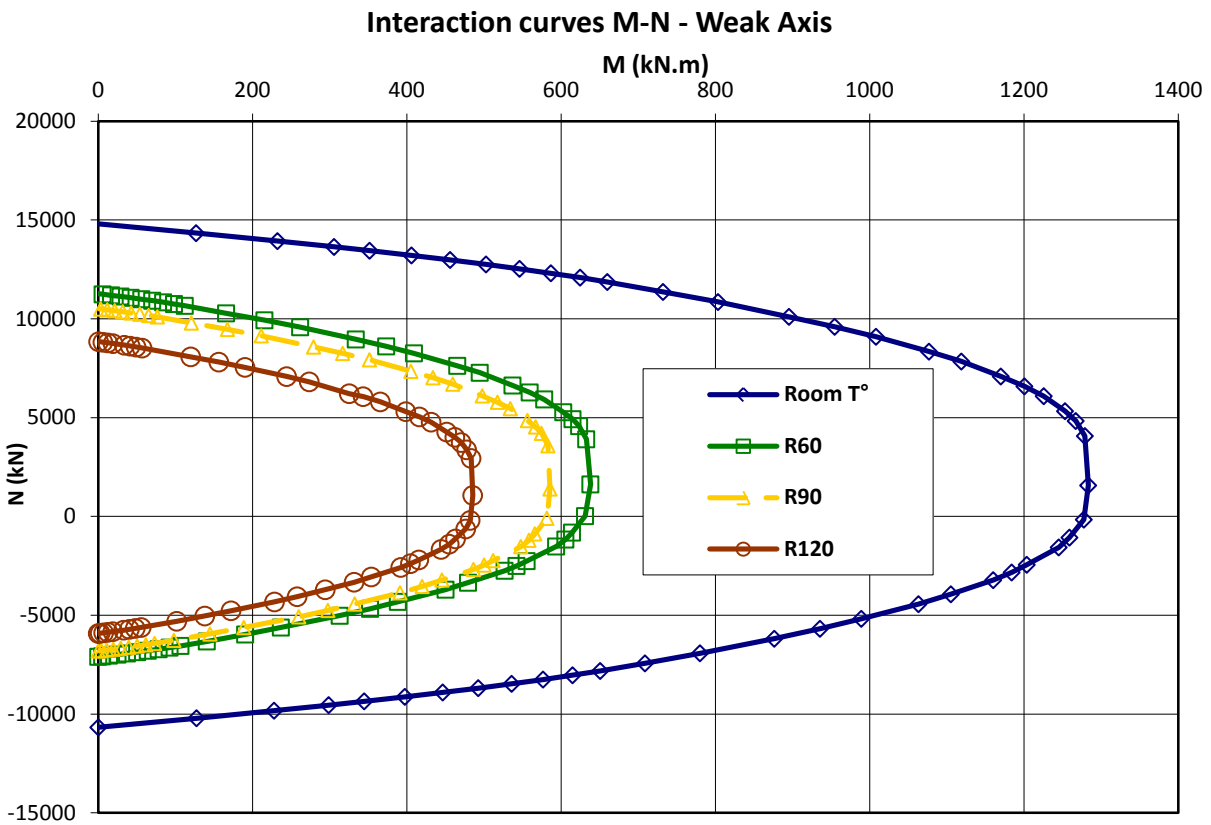


Figure 7: M-N interaction curves for a composite column at room and elevated temperatures

Effective flexural stiffness:

$$(EI)_{eff,z,120} = K_o (E_{a,120} I_{a,z} + K_{e,II} E_{cm,120} I_{c,z}) = 20963 \text{ kNm}^2$$

Coefficients K_o and $K_{e,II}$ are taken equal to 0.9 and 1.

Critical normal force for weak axis:

$$N_{cr,z,120} = \frac{\pi^2 * EI_{z,120}}{L^2} = 32581 \text{ kN}$$

Total eccentricity:

$$e = e_o + e_a = d/10 + L/500 = 40.6 + 16.8 = 61 \text{ mm}$$

The accidental eccentricity e_a is taken equal to $L/500$, instead of $L/1000$, in order to neglect the effect of residual stresses.

First-order design bending moment (acting in weak axis) under $N_{Sd} = 5000 \text{ kN}$ (assumption):

$$M_{z,Sd,1} = N_{Sd} * e = 305 \text{ kNm}$$

Factor accounting for second-order effects:

$$k = \frac{\beta}{1 - N_{Ed} / N_{cr,eff,z}} = 0.78 \geq 1 \rightarrow k = 1$$

Second-order design bending moment (acting in weak axis) under $N_{Sd} = 5000 \text{ kN}$ (assumption):

$$M_{z,Sd,2} = M_{z,Sd,1} * k = 305 \text{ kNm}$$

Plastic bending moment of the section under

$$N = N_{Sd}: M_{pl,N,Rd} = 401 \text{ kNm}$$

Resistance criteria of the section in weak axis:

$$\frac{M_{Ed,z}}{M_{pl,N,Rd,z}} = 0.76 \leq \alpha_M = 0.8$$

The coefficient α_M is kept equal to 0.9 for S235 and S355 steel grades and 0.8 for S420 and S460 steel grades at elevated temperatures.

The resistance criterion shows that the resistance of the composite columns with an eccentricity $e_o = d/10$ is higher than 5000 kN. An iterative procedure allows to determine $N_{Rd,120} = 5265 \text{ kN}$.

This method is not developed for sections with a diameter smaller than 400 mm. It was observed by use of SAFIR software that the influence of thermally-induced stresses on the fire resistance of the column was important and that the developed method could lead to unsafe predictions of the design resisting compression forces.

5 | Design Tables

5.1 | 406.4x8.8 Tube (S235 Steel Grade), HEM 200 encased profile (S355 Steel Grade)

Buckling Length $L = 2.35$ m			
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	8189
		R60	5932
		R90	5223
		R120	4403
C30/37	d/10	Cold	6580
		R60	3843
		R90	3357
		R120	2810
C30/37	d/5	Cold	4929
		R60	2507
		R90	2188
		R120	1826
C50/60	0	Cold	9441
		R60	7059
		R90	6139
		R120	5056
C50/60	d/10	Cold	7501
		R60	4640
		R90	3984
		R120	3243
C50/60	d/5	Cold	55853
		R60	3007
		R90	2573
		R120	2085

Buckling Length $L = 2.52$ m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	8113	8193	0.99
		R60	5880	5892	1.00
		R90	5175	5123	1.01
		R120	4363	4439	0.98
C30/37	d/10	Cold	6537	6744	0.97
		R60	3828	4152	0.92
		R90	3344	3684	0.91
		R120	2799	3258	0.86
C30/37	d/5	Cold	4902	5211	0.94
		R60	2499	3023	0.83
		R90	2180	2705	0.81
		R120	1819	2416	0.75

Buckling Length $L = 2.52$ m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C50/60	0	Cold	9344	9467	0.99
		R60	6983	7344	0.95
		R90	6067	6369	0.95
		R120	4998	5508	0.91
C50/60	d/10	Cold	7455	7786	0.96
		R60	4622	5241	0.88
		R90	3968	4583	0.87
		R120	3230	4006	0.81
C50/60	d/5	Cold	5549	5974	0.93
		R60	2996	3788	0.79
		R90	2565	3342	0.77
		R120	2077	2948	0.70

Buckling Length $L = 2.8$ m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	7985	8036	0.99
		R60	5765	5652	1.02
		R90	5073	4835	1.05
		R120	4277	4127	1.04
C30/37	d/10	Cold	6473	6626	0.98
		R60	3797	3925	0.97
		R90	3318	3444	0.96
		R120	2781	3014	0.92
C30/37	d/5	Cold	4855	5202	0.93
		R60	2486	2903	0.86
		R90	2168	2568	0.84
		R120	1810	2269	0.80
C50/60	0	Cold	9180	9291	0.99
		R60	6790	7068	0.96
		R90	5898	6022	0.98
		R120	4867	5101	0.95
C50/60	d/10	Cold	7378	7650	0.96
		R60	4382	4965	0.88
		R90	3782	4286	0.88
		R120	3107	3700	0.84
C50/60	d/5	Cold	5492	5966	0.92
		R60	2981	3642	0.82
		R90	2550	3169	0.80
		R120	2067	2762	0.75

Buckling Length L= 3.36 m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	7713	7692	1.00
	d/10	Cold	6343	6354	1.00
	d/5	Cold	4768	5122	0.93
C50/60	0	Cold	8828	8904	0.99
	d/10	Cold	7224	7336	0.98
	d/5	Cold	5385	5878	0.92

Buckling Length L= 3.6 m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	7590	7562	1.00
	d/10	Cold	6287	6227	1.01
	d/5	Cold	4730	5052	0.94
C50/60	0	Cold	8666	8726	0.99
	d/10	Cold	7157	7192	1.00
	d/5	Cold	5338	5798	0.92

Buckling Length L= 4 m					
406.4x8.8 Tube (S235), HEM 200 encased profile (S355)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	7372	7249	1.02
	d/10	Cold	6193	6006	1.03
	d/5	Cold	4665	4915	0.95
C50/60	0	Cold	8380	8410	1.00
	d/10	Cold	7052	6937	1.02
	d/5	Cold	5266	5638	0.93

5.2 | 406.4x8.8 Tube (S235 Steel Grade), HEB 200 encased profile (S355 Steel Grade)

Buckling Length $L = 2.35$ m			
406.4x88 Tube (S235), HEB 200 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	6811
		R60	4466
		R90	3936
		R120	3246
C30/37	d/10	Cold	5535
		R60	2885
		R90	2515
		R120	2047
C30/37	d/5	Cold	4226
		R60	1837
		R90	1591
		R120	1290
C50/60	0	Cold	8139
		R60	5652
		R90	4919
		R120	3962
C50/60	d/10	Cold	6511
		R60	3702
		R90	3180
		R120	2527
C50/60	d/5	Cold	4916
		R60	2355
		R90	2002
		R120	1569

Buckling Length $L = 2.52$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	6753	6784	1.00
		R60	4423	4576	0.97
		R90	3892	4008	0.97
		R120	3208	3453	0.93
C30/37	d/10	Cold	5504	5682	0.97
		R60	2873	3191	0.90
		R90	2505	2809	0.89
		R120	2039	2449	0.83
C30/37	d/5	Cold	4202	4506	0.93
		R60	1830	2279	0.80
		R90	1586	2024	0.78

Buckling Length $L = 2.52$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C50/60	0	Cold	8059	8118	0.99
		R60	5563	6121	0.91
		R90	4832	5339	0.91
		R120	3890	4643	0.84
C50/60	d/10	Cold	6472	6778	0.95
		R60	3567	4339	0.82
		R90	3064	3756	0.82
		R120	2445	3238	0.76
C50/60	d/5	Cold	4887	5322	0.92
		R60	2347	3067	0.77
		R90	1995	2677	0.75
		R120	1563	2329	0.67

Buckling Length $L = 2.8$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	6654	6666	1.00
		R60	4317	4394	0.98
		R90	3790	3787	1.00
		R120	3124	3192	0.98
C30/37	d/10	Cold	5453	5596	0.97
		R60	2775	3012	0.92
		R90	2414	2617	0.92
		R120	1971	2254	0.87
C30/37	d/5	Cold	4164	4498	0.93
		R60	1820	2188	0.83
		R90	1577	1920	0.82
		R120	1278	1672	0.76
C50/60	0	Cold	7926	7979	0.99
		R60	5316	5889	0.90
		R90	4607	5055	0.91
		R120	3721	4281	0.87
C50/60	d/10	Cold	6409	6674	0.96
		R60	3335	4106	0.81
		R90	2862	3501	0.82
		R120	2287	2975	0.77
C50/60	d/5	Cold	4842	5314	0.91
		R60	2333	2945	0.79
		R90	1982	2534	0.78
		R120	1553	2175	0.71

Buckling Length $L = 3.36$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	6447	6410	1.01
	d/10	Cold	5350	5394	0.99
	d/5	Cold	4090	4446	0.92
C50/60	0	Cold	7640	7478	1.02
	d/10	Cold	6282	6432	0.98
	d/5	Cold	4752	5254	0.90

Buckling Length $L = 3.6$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	6353	6294	1.01
	d/10	Cold	5306	5301	1.00
	d/5	Cold	4061	4398	0.92
C50/60	0	Cold	7510	7541	1.00
	d/10	Cold	6230	6318	0.99
	d/5	Cold	4715	5195	0.91

Buckling Length $L = 4$ m					
406.4x88 Tube (S235), HEB 200 encased profile (S355)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	6189	6087	1.02
	d/10	Cold	5235	5136	1.02
	d/5	Cold	4010	4298	0.93
C50/60	0	Cold	7279	7299	1.00
	d/10	Cold	6143	6119	1.00
	d/5	Cold	4654	5072	0.92

5.3 | 559x8.8 Tube (S235 Steel Grade), HD320x127 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
559x8.8 Tube (S235), HD320x127 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12527
		R60	9494
		R90	8811
		R120	7357
C30/37	d/10	Cold	9794
		R60	6342
		R90	5834
		R120	4810
C30/37	d/5	Cold	7370
		R60	4133
		R90	3785
		R120	3109
C50/60	0	Cold	15091
		R60	12209
		R90	11227
		R120	9257
C50/60	d/10	Cold	11712
		R60	8239
		R90	7494
		R120	6075
C50/60	d/5	Cold	8719
		R60	5321
		R90	4806
		R120	3874

Buckling Length $L = 2.52$ m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	13631	13439	1.01
		R60	10923	11705	0.93
		R90	10176	11105	0.92
		R120	8540	10468	0.82
C30/37	d/10	Cold	10245	10674	0.96
		R60	6851	8316	0.82
		R90	6337	7896	0.80
		R120	5265	7447	0.71
C30/37	d/5	Cold	7389	8024	0.92
		R60	4292	5611	0.76
		R90	3956	5375	0.74
		R120	3278	5120	0.64

Buckling Length $L = 2.52$ m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C50/60	0	Cold	16270	16110	1.01
		R60	13629	15170	0.90
		R90	12584	14338	0.88
		R120	10435	13506	0.77
C50/60	d/10	Cold	12041	12834	0.94
		R60	8631	10922	0.79
		R90	7884	10271	0.77
		R120	6447	9622	0.67
C50/60	d/5	Cold	8582	9568	0.90
		R60	5353	7322	0.73
		R90	4864	6925	0.70
		R120	3956	6542	0.60

Buckling Length $L = 2.8$ m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	13478	13238	1.02
		R60	10881	11524	0.94
		R90	10135	10878	0.93
		R120	8501	10176	0.84
C30/37	d/10	Cold	10158	10624	0.96
		R60	6818	8157	0.84
		R90	6304	7685	0.82
		R120	5239	7200	0.73
C30/37	d/5	Cold	7331	8018	0.91
		R60	4275	5596	0.76
		R90	3938	5356	0.74
		R120	3262	5088	0.64
C50/60	0	Cold	16067	15886	1.01
		R60	13536	14957	0.90
		R90	12490	14060	0.89
		R120	10355	13183	0.79
C50/60	d/10	Cold	11938	12778	0.93
		R60	8590	10770	0.80
		R90	7846	10044	0.78
		R120	6415	9342	0.69
C50/60	d/5	Cold	8514	9559	0.89
		R60	5329	7305	0.73
		R90	4842	6902	0.70
		R120	3940	6513	0.60

Buckling Length L= 3.36 m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	13163	12826	1.03
	d/10	Cold	9992	10424	0.96
	d/5	Cold	7216	8002	0.90
C50/60	0	Cold	15646	15422	1.01
	d/10	Cold	11736	12544	0.94
	d/5	Cold	8381	9538	0.88

Buckling Length L= 3.6 m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	13023	12646	1.03
	d/10	Cold	9920	10315	0.96
	d/5	Cold	7172	7990	0.90
C50/60	0	Cold	15457	15215	1.02
	d/10	Cold	11655	12416	0.94
	d/5	Cold	8325	9527	0.87

Buckling Length L= 4 m					
559x8.8 Tube (S235), HD320x127 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	12783	12340	1.04
	d/10	Cold	9804	10114	0.97
	d/5	Cold	7093	7972	0.89
C50/60	0	Cold	15131	14866	1.02
	d/10	Cold	11513	12176	0.95
	d/5	Cold	8230	9510	0.87

5.4 | 660x10 Tube (S235 Steel Grade), HD400x314 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
660x10 Tube (S235), HD400x314 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	22405
		R60	18945
		R90	18485
		R120	17908
C30/37	d/10	Cold	17432
		R60	13089
		R90	12721
		R120	12282
C30/37	d/5	Cold	13055
		R60	9020
		R90	8755
		R120	8420
C50/60	0	Cold	25999
		R60	22599
		R90	21843
		R120	20897
C50/60	d/10	Cold	19961
		R60	15673
		R90	15063
		R120	14330
C50/60	d/5	Cold	14852
		R60	10709
		R90	10270
		R120	9721

Buckling Length $L = 2.52$ m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	25548	25374	1.01
		R60	22530	23646	0.95
		R90	22064	22887	0.96
		R120	21482	21999	0.98
C30/37	d/10	Cold	18985	19103	0.99
		R60	14826	17068	0.87
		R90	14478	16674	0.87
		R120	14056	16206	0.87
C30/37	d/5	Cold	13645	13858	0.98
		R60	9842	12176	0.81
		R90	9602	11897	0.81
		R120	9306	11561	0.80

Buckling Length $L = 2.52$ m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C50/60	0	Cold	29086	28846	1.01
		R60	26187	28382	0.92
		R90	25423	27369	0.93
		R120	24467	26292	0.93
C50/60	d/10	Cold	21379	21952	0.97
		R60	17279	20521	0.84
		R90	16697	19857	0.84
		R120	15991	19188	0.83
C50/60	d/5	Cold	15248	15870	0.96
		R60	11370	14498	0.78
		R90	10971	14044	0.78
		R120	10488	13565	0.77

Buckling Length $L = 2.8$ m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	25294	24,994	1.01
		R60	22450	23402	0.96
		R90	21987	22589	0.97
		R120	21406	21632	0.99
C30/37	d/10	Cold	18850	19080	0.99
		R60	14775	17035	0.87
		R90	14428	16575	0.87
		R120	14002	16023	0.87
C30/37	d/5	Cold	13557	13850	0.98
		R60	9808	12160	0.81
		R90	9568	11876	0.81
		R120	9273	11536	0.80
C50/60	0	Cold	28775	28479	1.01
		R60	26107	28114	0.93
		R90	25341	27025	0.94
		R120	24380	25898	0.94
C50/60	d/10	Cold	21218	21926	0.97
		R60	17219	20489	0.84
		R90	16639	19794	0.84
		R120	15929	19031	0.84
C50/60	d/5	Cold	15148	15854	0.96
		R60	11330	14476	0.78
		R90	10933	14021	0.78
		R120	10457	13538	0.77

Buckling Length L= 3.36 m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	24775	24254	1.02
	d/10	Cold	18578	18907	0.98
	d/5	Cold	13381	13826	0.97
C50/60	0	Cold	28137	27726	1.01
	d/10	Cold	20911	21750	0.96
	d/5	Cold	14949	15823	0.94

Buckling Length L= 3.6 m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	24546	23936	1.03
	d/10	Cold	18463	18774	0.98
	d/5	Cold	13299	13814	0.96
C50/60	0	Cold	27855	27394	1.02
	d/10	Cold	20781	21599	0.96
	d/5	Cold	14856	15810	0.94

Buckling Length L= 4 m					
660x10 Tube (S235), HD400x314 encased profile (S460M)					
Concrete	Excentricity	Fire	N _{rd} (kN) simplified method	Safir (kN)	Ratio
C30/37	0	Cold	24156	23406	1.03
	d/10	Cold	18273	18500	0.99
	d/5	Cold	13177	13794	0.96
C50/60	0	Cold	27371	26831	1.02
	d/10	Cold	20566	21294	0.97
	d/5	Cold	14718	15783	0.93

5.5 | 508x8.8 Tube (S235 Steel Grade), HEM 260 encased profile (S355 Steel Grade)

Buckling Length $L = 2.35$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12820
		R60	10397
		R90	9604
		R120	8081
C30/37	d/10	Cold	10083
		R60	6862
		R90	6299
		R120	5264
C30/37	d/5	Cold	7476
		R60	4544
		R90	4160
		R120	3472
C50/60	0	Cold	14872
		R60	12434
		R90	11373
		R120	9439
C50/60	d/10	Cold	11565
		R60	8286
		R90	7509
		R120	6159
C50/60	d/5	Cold	8510
		R60	5459
		R90	4929
		R120	4032

Buckling Length $L = 2.52$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12725
		R60	9765
		R90	9421
		R120	8993
C30/37	d/10	Cold	9594
		R60	6302
		R90	6020
		R120	5698
C30/37	d/5	Cold	7074
		R60	4092
		R90	3904
		R120	3683

Buckling Length $L = 2.52$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	14751
		R60	11832
		R90	11268
		R120	10573
C50/60	d/10	Cold	11082
		R60	7755
		R90	7288
		R120	6749
C50/60	d/5	Cold	8126
		R60	5013
		R90	4694
		R120	4325

Buckling Length $L = 2.8$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12565
		R60	9718
		R90	9367
		R120	8931
C30/37	d/10	Cold	9509
		R60	6272
		R90	5991
		R120	5671
C30/37	d/5	Cold	7019
		R60	4075
		R90	3888
		R120	3667
C50/60	0	Cold	14547
		R60	11728
		R90	11161
		R120	10463
C50/60	d/10	Cold	10983
		R60	7716
		R90	7251
		R120	6714
C50/60	d/5	Cold	8058
		R60	4989
		R90	4674
		R120	4306

Buckling Length $L = 3.36$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12234
	d/10	Cold	9342
	d/5	Cold	6907
C50/60	0	Cold	14122
	d/10	Cold	10788
	d/5	Cold	7925

Buckling Length $L = 3.6$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	12086
	d/10	Cold	9273
	d/5	Cold	6862
C50/60	0	Cold	13930
	d/10	Cold	10708
	d/5	Cold	7868

Buckling Length $L = 4$ m			
508x8.8 Tube (S235), HEM 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	11829
	d/10	Cold	9162
	d/5	Cold	6786
C50/60	0	Cold	13596
	d/10	Cold	10579
	d/5	Cold	7776

5.6 | 508x8.8 Tube (S235 Steel Grade), HEB 260 encased profile (S355 Steel Grade)

Buckling Length $L = 2.35$ m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9996
		R60	7255
		R90	6781
		R120	5591
C30/37	d/10	Cold	7978
		R60	4776
		R90	4416
		R120	3595
C30/37	d/5	Cold	6035
		R60	3075
		R90	2828
		R120	2285
C50/60	0	Cold	12191
		R60	9447
		R90	8728
		R120	7101
C50/60	d/10	Cold	9552
		R60	6301
		R90	5735
		R120	4586
C50/60	d/5	Cold	7146
		R60	4017
		R90	3629
		R120	2874

Buckling Length $L = 2.52$ m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9927
		R60	6866
		R90	6528
		R120	6111
C30/37	d/10	Cold	7683
		R60	4450
		R90	4172
		R120	3832
C30/37	d/5	Cold	5812
		R60	2806
		R90	2615
		R120	2391

Buckling Length L= 2.52 m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	12097
		R60	9043
		R90	8480
		R120	7790
C50/60	d/10	Cold	9273
		R60	5990
		R90	5528
		R120	4961
C50/60	d/5	Cold	6921
		R60	3750
		R90	3429
		R120	3056

Buckling Length L= 2.8 m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9813
		R60	6820
		R90	6476
		R120	6051
C30/37	d/10	Cold	7625
		R60	4426
		R90	4149
		R120	3811
C30/37	d/5	Cold	5766
		R60	2792
		R90	2603
		R120	2380
C50/60	0	Cold	11939
		R60	8938
		R90	8371
		R120	7676
C50/60	d/10	Cold	9199
		R60	5954
		R90	5485
		R120	4929
C50/60	d/5	Cold	6865
		R60	3731
		R90	3411
		R120	3042

Buckling Length $L = 3.36$ m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9578
	d/10	Cold	7509
	d/5	Cold	5680
C50/60	0	Cold	11612
	d/10	Cold	9048
	d/5	Cold	6760

Buckling Length $L = 3.6$ m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9474
	d/10	Cold	7461
	d/5	Cold	5642
C50/60	0	Cold	11465
	d/10	Cold	8986
	d/5	Cold	6717

Buckling Length $L = 4$ m			
508x8.8 Tube (S235), HEB 260 encased profile (S355)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	9294
	d/10	Cold	7377
	d/5	Cold	5582
C50/60	0	Cold	11210
	d/10	Cold	8880
	d/5	Cold	6642

5.7 | 559x8.8 Tube (S235 Steel Grade), HD320x245 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	18903
		R60	16602
		R90	15184
		R120	13046
	d/10	Cold	13927
		R60	10537
		R90	9588
		R120	8210
	d/5	Cold	9854
		R60	6828
		R90	6212
		R120	5308
C50/60	0	Cold	21360
		R60	19081
		R90	17341
		R120	14715
C50/60	d/10	Cold	15606
		R60	12190
		R90	10985
		R120	9262
C50/60	d/5	Cold	10985
		R60	7842
		R90	7070
		R120	5945

Buckling Length $L = 2.52$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	18762
		R60	16559
		R90	15142
		R120	13010
C30/37	d/10	Cold	13851
		R60	10507
		R90	9561
		R120	8187
C30/37	d/5	Cold	9808
		R60	6811
		R90	6197
		R120	5295

Buckling Length $L = 2.52$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	21189
		R60	19037
		R90	17298
		R120	14675
C50/60	d/10	Cold	15514
		R60	12151
		R90	10954
		R120	9236
C50/60	d/5	Cold	10933
		R60	7823
		R90	7052
		R120	5930

Buckling Length $L = 2.8$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	18526
		R60	16485
		R90	15074
		R120	12952
C30/37	d/10	Cold	13724
		R60	10457
		R90	9515
		R120	8148
C30/37	d/5	Cold	9732
		R60	6786
		R90	6174
		R120	5275
C50/60	0	Cold	20903
		R60	18964
		R90	17227
		R120	14608
C50/60	d/10	Cold	15372
		R60	12094
		R90	10902
		R120	9191
C50/60	d/5	Cold	10842
		R60	7794
		R90	7022
		R120	5904

Buckling Length $L = 3.36$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	18038
	d/10	Cold	13474
	d/5	Cold	9580
C50/60	0	Cold	20305
	d/10	Cold	15092
	d/5	Cold	10659

Buckling Length $L = 3.6$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	17819
	d/10	Cold	13373
	d/5	Cold	9513
C50/60	0	Cold	20037
	d/10	Cold	14972
	d/5	Cold	10579

Buckling Length $L = 4$ m			
559x8.8 Tube (S235), HD320x245 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	17440
	d/10	Cold	13200
	d/5	Cold	9407
C50/60	0	Cold	19569
	d/10	Cold	14783
	d/5	Cold	10453

5.8 | 610x10 Tube (S235 Steel Grade), HD360x134 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	15852
		R60	12243
		R90	11398
		R120	9594
C30/37	d/10	Cold	11983
		R60	8024
		R90	7429
		R120	6211
C30/37	d/5	Cold	8836
		R60	5187
		R90	4786
		R120	3990
C50/60	0	Cold	19100
		R60	15536
		R90	14350
		R120	11951
C50/60	d/10	Cold	14154
		R60	10211
		R90	9355
		R120	7696
C50/60	d/5	Cold	10307
		R60	6494
		R90	5923
		R120	4854

Buckling Length $L = 2.52$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	157600
		R60	12218
		R90	11374
		R120	9571
C30/37	d/10	Cold	11929
		R60	8003
		R90	7409
		R120	6194
C30/37	d/5	Cold	8798
		R60	5175
		R90	4774
		R120	3980

Buckling Length $L = 2.52$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	18978
		R60	15506
		R90	14321
		R120	11925
C50/60	d/10	Cold	14090
		R60	10187
		R90	9333
		R120	7671
C50/60	d/5	Cold	10257
		R60	6478
		R90	5908
		R120	4842

Buckling Length $L = 2.8$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	15609
		R60	12177
		R90	11334
		R120	9534
C30/37	d/10	Cold	11838
		R60	7972
		R90	7377
		R120	6167
C30/37	d/5	Cold	1741
		R60	5153
		R90	4757
		R120	3966
C50/60	0	Cold	18775
		R60	15457
		R90	14274
		R120	11882
C50/60	d/10	Cold	13987
		R60	10148
		R90	9293
		R120	7638
C50/60	d/5	Cold	10183
		R60	6455
		R90	5883
		R120	4824

Buckling Length $L = 3.36$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	15300
	d/10	Cold	11669
	d/5	Cold	8625
C50/60	0	Cold	18359
	d/10	Cold	13780
	d/5	Cold	10041

Buckling Length $L = 3.6$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	15164
	d/10	Cold	11599
	d/5	Cold	8576
C50/60	0	Cold	18175
	d/10	Cold	13696
	d/5	Cold	9977

Buckling Length $L = 4$ m			
610x10 Tube (S235), HD360x134 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	14933
	d/10	Cold	11480
	d/5	Cold	8498
C50/60	0	Cold	17859
	d/10	Cold	13553
	d/5	Cold	9878

5.9 | 610x10 Tube (S235 Steel Grade), HD360x162 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	17168
		R60	13685
		R90	12691
		R120	10738
	d/10	Cold	12931
		R60	8987
		R90	8294
		R120	6982
	d/5	Cold	9491
		R60	5858
		R90	5397
		R120	4530
C50/60	0	Cold	20364
		R60	16923
		R90	15578
		R120	13039
C50/60	d/10	Cold	15073
		R60	11142
		R90	10185
		R120	8444
C50/60	d/5	Cold	10932
		R60	7168
		R90	6528
		R120	5392

Buckling Length $L = 2.8$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	16898
		R60	13607
		R90	12615
		R120	10672
C30/37	d/10	Cold	12778
		R60	8929
		R90	8243
		R120	6936
C30/37	d/5	Cold	9388
		R60	5824
		R90	5365
		R120	4503

Buckling Length $L = 2.52$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	20231
		R60	16890
		R90	15548
		R120	13010
C50/60	d/10	Cold	15009
		R60	11117
		R90	10158
		R120	8424
C50/60	d/5	Cold	10879
		R60	7151
		R90	6513
		R120	5382

Buckling Length $L = 2.8$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	16898
		R60	13607
		R90	12615
		R120	10672
C30/37	d/10	Cold	12778
		R60	8929
		R90	8243
		R120	6936
C30/37	d/5	Cold	9388
		R60	5824
		R90	5365
		R120	4503
C50/60	0	Cold	20011
		R60	16835
		R90	15494
		R120	12961
C50/60	d/10	Cold	14893
		R60	11070
		R90	10119
		R120	8384
C50/60	d/5	Cold	10805
		R60	7126
		R90	6489
		R120	5359

Buckling Length $L = 3.36$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	16554
	d/10	Cold	12594
	d/5	Cold	9266
C50/60	0	Cold	19559
	d/10	Cold	14677
	d/5	Cold	10647

Buckling Length $L = 3.6$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	16403
	d/10	Cold	12518
	d/5	Cold	9213
C50/60	0	Cold	19359
	d/10	Cold	14587
	d/5	Cold	10584

Buckling Length $L = 4$ m			
610x10 Tube (S235), HD360x162 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	16145
	d/10	Cold	12388
	d/5	Cold	9118
C50/60	0	Cold	19015
	d/10	Cold	14434
	d/5	Cold	10478

5.10 | 660x10 Tube (S235 Steel Grade), HD400x216 encased profile (S460M Steel Grade)

Buckling Length $L = 2.35$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	21130
		R60	17580
		R90	17119
		R120	16538
C30/37	d/10	Cold	15773
		R60	11510
		R90	11159
		R120	10730
C30/37	d/5	Cold	11453
		R60	7523
		R90	7280
		R120	6995
C50/60	0	Cold	24874
		R60	21420
		R90	20666
		R120	19722
C50/60	d/10	Cold	18279
		R60	14082
		R90	13500
		R120	12784
C50/60	d/5	Cold	13116
		R60	9103
		R90	8705
		R120	8219

Buckling Length $L = 2.52$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	21032
		R60	17547
		R90	17085
		R120	16501
C30/37	d/10	Cold	15701
		R60	11483
		R90	11133
		R120	10705
C30/37	d/5	Cold	11403
		R60	7506
		R90	7267
		R120	6978

Buckling Length $L = 2.52$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C50/60	0	Cold	24721
		R60	21382
		R90	20627
		R120	19683
C50/60	d/10	Cold	18195
		R60	14055
		R90	13469
		R120	12749
C50/60	d/5	Cold	13064
		R60	9087
		R90	8685
		R120	8205

Buckling Length $L = 2.8$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	20813
		R60	17492
		R90	17029
		R120	16441
C30/37	d/10	Cold	15596
		R60	11439
		R90	11095
		R120	10667
C30/37	d/5	Cold	11326
		R60	7479
		R90	7242
		R120	6953
C50/60	0	Cold	24467
		R60	21320
		R90	20564
		R120	19614
C50/60	d/10	Cold	18072
		R60	14001
		R90	13417
		R120	12694
C50/60	d/5	Cold	12974
		R60	9054
		R90	8654
		R120	8175

Buckling Length $L = 3.36$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	20411
	d/10	Cold	15381
	d/5	Cold	11177
C50/60	0	Cold	23947
	d/10	Cold	17821
	d/5	Cold	12800

Buckling Length $L = 3.6$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	20235
	d/10	Cold	15293
	d/5	Cold	11116
C50/60	0	Cold	23711
	d/10	Cold	17711
	d/5	Cold	12736

Buckling Length $L = 4$ m			
660x10 Tube (S235), HD400x216 encased profile (S460M)			
Concrete	Excentricity	Fire	N_{rd} (kN) simplified method
C30/37	0	Cold	19935
	d/10	Cold	15144
	d/5	Cold	11012
C50/60	0	Cold	23325
	d/10	Cold	17531
	d/5	Cold	12613

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